STORMWATER REPORT

PROPOSED SINGLE FAMILY RESIDENCE

25 HENRY STREET ARLINGTON, MASSACHUSETTS

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January 29, 2021

<u>Narrative</u>

1.0 Introduction and Background

The following report details a storm water analysis completed for a proposed singlefamily home site located on Henry Street in Arlington, Massachusetts (refer to Figure 1: USGS Locus Map). The change to the property will result in an increase in impervious surfaces by more than 350 s.f. and is therefore required to meet Arlington's Storm Water Mitigation Bylaw (Art. 15). This study will provide a comparative analysis of the hydrologic characteristics of the pre- vs. post-development conditions of the site and provide information on the rate of surface water runoff generated by the project.

2.0 Existing Conditions

The site is located on the east side of the public roadway Henry St. approximately 200 feet north of Elwern St. The property is bounded by Henry St. to the west and residential use properties to the north, east, and south. The subject property is located in the R-1 zoning district and has a land area of 6,441 square feet. The property is primarily undeveloped and was recently subdivided from 29 Henry St. There is presently approximately 250 s.f of impervious surfaces from an existing shed and pavement. The slopes on the property range between 10 and 60 percent with the grades dropping from the highest elevations at the northeast boundary to the western boundaries Henry St. Figure 2 represents the existing topographic conditions at the site.

Based on information available from the USDA Conservation Service the underlying soils at the site are those typical of the Charlton Urban Land Complex. These soils are considered to be in the Hydrologic Soil Group "A". Information on the site's soils has been included in the report's appendix.

3.0 Project Description

The project proposes the construction of a 32' by 44' (1,450 s.f.) single family dwelling. Other impervious surfaces that will be created as part of the development includes a walk and driveway to access a two-car garage. When completed the new building site will have approximately 2,400 s.f. of impervious surfaces. The project will also include two stormwater recharge systems. The proposed dwelling will include a roof drainage system of roof gutters and downspouts. The roof drainage will be directly piped to a storm water recharge system. The driveway will be constructed with a trench grate connected to a second recharge system. Information on the site development and the design of the recharge system has been included in the appendix of this report.

4.0 Peak Flow Runoff Rates

Peak flow rates were studied under existing and proposed conditions for the 2, 10, 25, & 100 year, 24-hour Type III storm events. Two points of comparisons or study point (as described below in Table 1 and as shown in Figures 2 and 3) were developed and studied in an effort to micro-analyze runoff rates to the abutting properties and roadway. The study points were consistent between pre- and post-development conditions.

TABLE 1: STUDY POINT OF COMPARISON

Study Point	Description
Α	Flows from the site to Henry St.

For both pre- and post-development conditions, the site was divided into subcatchment areas based on topography, drainage patterns, and "POC's". One subcatchment (E-1) was utilized to study pre-development conditions, and one subcatchments (P-1) was utilized to study post-development conditions (refer to Figures 2 and 3, in the report's appendix). Subcatchment P-2 & P-3 were utilized to determine the performance of the stormwater recharge systems. These subcatchments were made up of the impervious surfaces of the dwelling's roof and driveway that will drain to the recharge systems.

Technical Release 55 (TR-55) was utilized to obtain weighted curve numbers (CNs) for each of the pre- and post-development subcatchment areas. Inputs for obtaining the weighted CNs were based on ground cover type and hydrologic soil group (HSG) soils classified by United States Department of Agriculture (USDA) soil survey data. Supporting information on the weighted curve numbers has been included in the report's appendix.

TR-55 was also utilized to obtain times of concentration (TCs) for each of the pre- and post-development subcatchment areas. Various flow paths were evaluated for each subcatchment area to determine the most hydrologically remote point within that subcatchment, which did not necessarily correspond to the longest flow path. Flow paths generally include a segment of sheet flow of up to 100 feet, segment(s) of shallow concentrated flow, and in some cases segment(s) of channel flow, with the sheet flow having the greatest impact on the TC. For each segment of flow, the length, slope, and cover type were entered to calculate the TC. The same methodology for calculating TC

was employed consistently between pre-and post development conditions. Refer to the attached TR-55 TC data. Since such small watersheds makes up the study areas a 6 minimum time of concentration was assumed for both the pre and post development condition.

CNs and TCs obtained from TR-55 were input into the *Hydraflow*[®] Hydrographs software package, which utilizes the National Resources Conservation Service (NRCS) (formerly "SCS") method to generate and route hydrographs. The resulting hydrographs have been included in the report's appendix.

The resulting analysis determines flows to the "POC's", as the post-development peak runoff rates do not exceed the pre-development peak runoff rates for the design storms. Table 2 (below) summarizes the attached *Hydraflow*[®] Hydrographs analysis.

Design Poin	nt		Peak Flows (CFS) Storm Events					
		<u>2 YR</u>	<u>10 YR</u>	<u>25 YR</u>	<u>100 YR</u>			
А	Existing	0.0	0.0	0.0	0.1			
	Proposed	0.0	0.0	0.0	0.3			

TABLE 2: RUNOFF COMPARISON

10.0 Conclusion

Based on the analysis performed the storm water mitigation (recharge systems) proposed will capture the additional stormwater generated from the proposed construction and their will be no increase in the storm water runoff from the site.







APPENDIX

SITE SOIL INFORMATION CURVE NUMBER COMPUTATIONS HYDROGRAPH REPORTS RECHARGE AREA CHARACTERISTICS STORMWATER MANAGEMENT PLAN SITE SOIL INFORMATION



USDA Natural Resources Conservation Service Web Soil Survey National Cooperative Soil Survey



Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
629C	Canton-Charlton-Urban land complex, 3 to 15 percent slopes	A	0.6	45.2%
631C	Charlton-Urban land- Hollis complex, 3 to 15 percent slopes, rocky	A	0.7	54.8%
Totals for Area of Inter	est	1.2	100.0%	

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition Component Percent Cutoff: None Specified Tie-break Rule: Higher



Middlesex County, Massachusetts

631C—Charlton-Urban land-Hollis complex, 3 to 15 percent slopes, rocky

Map Unit Setting

National map unit symbol: vr1g Elevation: 0 to 1,000 feet Mean annual precipitation: 32 to 54 inches Mean annual air temperature: 43 to 54 degrees F Frost-free period: 110 to 240 days Farmland classification: Not prime farmland

Map Unit Composition

Charlton and similar soils: 45 percent Urban land: 35 percent Hollis and similar soils: 10 percent Minor components: 10 percent Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Charlton

Setting

Landform: Drumlins, ground moraines Landform position (two-dimensional): Backslope Landform position (three-dimensional): Side slope Down-slope shape: Linear Across-slope shape: Convex Parent material: Friable loamy eolian deposits over friable loamy basal till derived from granite and gneiss

Typical profile

H1 - 0 to 5 inches: fine sandy loam

H2 - 5 to 22 inches: sandy loam

H3 - 22 to 65 inches: gravelly sandy loam

Properties and qualities

Slope: 3 to 15 percent Depth to restrictive feature: More than 80 inches Drainage class: Well drained Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.60 to 6.00 in/hr) Depth to water table: More than 80 inches Frequency of flooding: None Frequency of ponding: None Available water capacity: Moderate (about 7.3 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 3e Hydrologic Soil Group: A

USDA

Ecological site: F144AY034CT - Well Drained Till Uplands *Hydric soil rating:* No

Description of Urban Land

Setting

Landform position (two-dimensional): Footslope Landform position (three-dimensional): Base slope Down-slope shape: Linear Across-slope shape: Linear Parent material: Excavated and filled land

Description of Hollis

Setting

Landform: Ridges, hillslopes Landform position (two-dimensional): Backslope Landform position (three-dimensional): Side slope Down-slope shape: Linear Across-slope shape: Convex Parent material: Friable, shallow loamy basal till over granite and gneiss

Typical profile

H1 - 0 to 2 inches: fine sandy loam *H2 - 2 to 14 inches:* fine sandy loam

H3 - 14 to 18 inches: unweathered bedrock

Properties and qualities

Slope: 3 to 15 percent
Surface area covered with cobbles, stones or boulders: 9.0 percent
Depth to restrictive feature: 8 to 20 inches to lithic bedrock
Drainage class: Well drained
Capacity of the most limiting layer to transmit water (Ksat): Very low to moderately low (0.00 to 0.14 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water capacity: Very low (about 2.0 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 6s Hydrologic Soil Group: D Ecological site: F144AY033MA - Shallow Dry Till Uplands Hydric soil rating: No

Minor Components

Canton

Percent of map unit: 4 percent Landform: Hills Landform position (two-dimensional): Backslope, toeslope Landform position (three-dimensional): Side slope, base slope

JSDA

Down-slope shape: Linear *Across-slope shape:* Convex *Hydric soil rating:* No

Udorthents, loamy

Percent of map unit: 2 percent Hydric soil rating: No

Rock outcrop

Percent of map unit: 2 percent Landform: Ledges Landform position (two-dimensional): Summit Landform position (three-dimensional): Head slope Down-slope shape: Concave Across-slope shape: Concave

Scituate

Percent of map unit: 1 percent Landform: Depressions, hillslopes Landform position (two-dimensional): Toeslope, summit Landform position (three-dimensional): Base slope, head slope Down-slope shape: Linear Across-slope shape: Concave Hydric soil rating: No

Montauk

Percent of map unit: 1 percent Landform: Hillslopes Landform position (two-dimensional): Shoulder, summit Landform position (three-dimensional): Nose slope, head slope Down-slope shape: Convex Across-slope shape: Convex Hydric soil rating: No

Data Source Information

Soil Survey Area: Middlesex County, Massachusetts Survey Area Data: Version 20, Jun 9, 2020



Commonwealth of Massachusetts City/Town of April Name Form 11 - Soil Suitability Assessment

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C. On-Site Review (continued)

Deep Observation Hole Number:

- 20

Other HEM Consistence (Moist) 4 メト Soil Arr. <u>E</u> Structure 17 J Soil Cobbles & Stones С V 000 Coarse Fragments % by Volume 5 Gravel 6 S Soil Texture (USDA) シビ Ч N こ Percent Redoximorphic Features (mottles) Color Depth 1 \ ١ Depth (in.) Soil Horizon/ Soil Matrix: Color-Layer Moist (Munsell) 10/24/3 Additional Notes: þ 4 9 3

TOW Day, Daw () Ripobsvid by The Barrows, date: 11/5 Refised of bat. of hole (weathered - Materal : 1 Refisal a ror

Form 11 – Soil Suitability Assessment

CURVE NUMBER COMPUTATIONS

Project 25 HE	NRY ST.	By 7B Date 1/29/2				
Location ARLINE	aton	Checked Date				
Check one: Prese	nt Developed	/				
1. Runoff curve n	umber					
Soil name and	Cover description		CI	N ¹ /	Area	Product of
group		Jilion manage	2-2	2-4	acres	CIV X died
(appendix A)	(cover type, treatment, and nydrologic cond impervious; unconnected/connected imper	vious area ratio)	Table 2	Figure	□mi ² □%	
	IMPERVIOU	S	98		0.01	0,98
A	OPEN SPACE ((4000)	39		0.14	5.46
¹ / Use only one CN source	e per line		Tot		DIE	6,44
		4	101		2113	¢.,,,
CN (weighted) = <u>total</u> tota	$\frac{\text{product}}{\text{al area}} = \frac{6.44}{0.15} = -$	42.9	Use C	N 🗭 [43	
2. Runoff						
		Storm #1	S	torm #2		Storm #3
Frequency	yr					
Rainfall, P	(24-hour) in					
Runoff, Q (Use P and	d CN with table 2-1, figure 2-1, or					
equation 2	2-3 and 2-4)					

Project 25/	LENRY ST		Ву	3			Date	19/21
Location	RLINGTON	V	Checked				Date	1
Check one:	Present Developed	110-	1 "					
1. Runoff curv	/e number							
Soil name and	c	Cover description			CN ¹	/	Area	Product of
group	(cover type, treatme	nt, and hydrologic cond	dition; percent	9 2-2	e 2-3	re 2-4	acres	ON X died
(appendix A)	impervious; unconne	ected/connected imperv	vious area ratio)	Table	Figur	Ligu		
	IMPE	EVIOUS	2	98		¢	0,005	0.49
4	OPENS	PALE (E	haco)	39		C	5,10	3,90
								1
-								
1/ Use only one CN	source per line			1	fotals	s 📦 🤅	0,105	4:39
CN (weighted) = _	$\frac{\text{total product}}{\text{total area}} = \frac{4}{0}$	1.39 = - 105	41.8	Use	CN	•	42	2
2. Runoff								
			Storm #1		Storr	m #2		Storm #3
Frequ	iency	yr						
Rainfa	all, P (24-hour)	in						
Runo	ff, Q	in						
equa	ation 2-3 and 2-4)	∠-1, UI						

Project Z4	SHEN	IRY ST		By JB				Date	29/21
Location	RUN	GTON		Checked				Date	
Check one:	Present	Developed	P-2	2" (2	DEI	VEI	NA	N	
1. Runoff c	urve nun	nber							
Soil name and		Cover d	escription			CN ¹		Area	Product of
hydrologic group					Q	e S	2-4	Macres	CN x area
(appendix A)		(cover type, treatment, and h impervious; unconnected/cor	ydrologic condi nnected impervi	tion; percent ous area ratio)	Table 2-	Figure 2	Figure 2	— mi ² □ %	
		IMPERV	>	98			0.015		
¹ / Use only on	e CN source per	rline			1	otal	s 🝺	0:013	
CN (weighted)	total pro	nduct						04	
	total a	rea			Use	CN		78	2
2. Runoff			F					1	
			_	Storm #1		Stor	m #2		Storm #3
F	requency		yr						
R	lainfall, P (24	-hour)	in						
R	Runoff, Q (Use P and CN	with table 2-1, figure 2-1, or	in						
	equation 2-3 a	nd 2-4)							

Project 25	HEN	EY ST.		By 7B Date 1/29/2				29/21	
Location A-1	RLIN	GTON		Checked Date					
Check one:	Present	Developed	'P-:	3" (Roc	PE)		
1. Runoff cu	rve num	iber							
Soil name and		Cover descr	iption			CN ¹	J	Area	Product of
group					2-2	2-3	2-4	Macres	UN X area
(appendix A)		(cover type, treatment, and hydrol impervious; unconnected/connect	ogic condi ed imperv	ition; percent ious area ratio)	Table 2	Figure	Figure	□ mi ² □ %	
					98			0.03	
1/ Use only one C	CN source per	line			 1	otal	s 🕩	0.03	
CN (weighted) =	total pro	rea	_=_	;	Use	CN		98	5
2. Runoff									
			_	Storm #1		Stori	m #2		Storm #3
Free	quency		yr						
Raii	nfall, P (24·	hour)	in						
Run (U	noff, Q Jse P and CN	with table 2-1, figure 2-1, or	in						
ec	quation 2-3 ar	nd 2-4)							

HYDROGRAPH REPORTS

Hydrograph Return Period Recap

Hydraflow Hydrographs by Intelisolve v9.2

Hyd.	yd. Hydrograph Inflow Peak Outflow (cfs)								Hydrograph		
No.	type (origin)	Hyd(s)	1-Yr	2-Yr	3-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr	description
1	SCS Runoff			0.001			0.010	0.030		0.135	E-1
2	SCS Runoff			0.000			0.005	0.017		0.082	P-1
3	SCS Runoff			0.047			0.068	0.081		0.107	P-2
4	SCS Runoff			0.093			0.136	0.161		0.213	P-3
5	Reservoir	3		0.000			0.000	0.000		0.005	Chamber Sys A
6	Reservoir	4		0.000			0.000	0.019		0.221	Chamber Sys B
7	Combine	2, 5, 6		0.000			0.005	0.031		0.301	Flow to SP-A

Hydrograph Summary Report

Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph description
1	SCS Runoff	0.001	1	1324	8				E-1
2	SCS Runoff	0.000	1	1324	3				P-1
3	SCS Runoff	0.047	1	724	161				P-2
4	SCS Runoff	0.093	1	724	322				P-3
5	Reservoir	0.000	1	655	0	3	209.20	56.2	Chamber Sys A
6	Reservoir	0.000	1	605	0	4	229.47	126	Chamber Sys B
7	Combine	0.000	1	1324	3	2, 5, 6			Flow to SP-A
hydro Henry St.gpw					Return P	eriod: 2 Ye	ar	Monday, Fe	eb 1, 2021

Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 1

E-1

Hydrograph type	= SCS Runoff	Peak discharge	= 0.001 cfs
Storm frequency	= 2 yrs	Time to peak	= 1324 min
Time interval	= 1 min	Hyd. volume	= 8 cuft
Drainage area	= 0.150 ac	Curve number	= 43
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= USER	Time of conc. (Tc)	= 6.00 min
Total precip.	= 3.10 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484



Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 2

P-1

Hydrograph type	= SCS Runoff	Peak discharge	= 0.000 cfs
Storm frequency	= 2 yrs	Time to peak	= 1324 min
Time interval	= 1 min	Hyd. volume	= 3 cuft
Drainage area	= 0.105 ac	Curve number	= 42
Basin Šlope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= USER	Time of conc. (Tc)	= 6.00 min
Total precip.	= 3.10 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484



Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 3

P-2

Hydrograph type	= SCS Runoff	Peak discharge	= 0.047 cfs
Storm frequency	= 2 yrs	Time to peak	= 724 min
Time interval	= 1 min	Hyd. volume	= 161 cuft
Drainage area	= 0.015 ac	Curve number	= 98
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= USER	Time of conc. (Tc)	= 6.00 min
Total precip.	= 3.10 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484



Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 4

P-3

Hydrograph type	= SCS Runoff	Peak discharge	= 0.093 cfs
Storm frequency	= 2 yrs	Time to peak	= 724 min
Time interval	= 1 min	Hyd. volume	= 322 cuft
Drainage area	= 0.030 ac	Curve number	= 98
Basin Šlope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= USER	Time of conc. (Tc)	= 6.00 min
Total precip.	= 3.10 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484



Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 5

Chamber Sys A

Hydrograph type	= Reservoir	Peak discharge	= 0.000 cfs
Storm frequency	= 2 yrs	Time to peak	= 655 min
Time interval	= 1 min	Hyd. volume	= 0 cuft
Inflow hyd. No.	= 3 - P-2	Max. Elevation	= 209.20 ft
Reservoir name	= RA-B	Max. Storage	= 56 cuft

Storage Indication method used. Exfiltration extracted from Outflow.



Pond Report

Hydraflow Hydrographs by Intelisolve v9.2

Pond No. 1 - RA-B

Pond Data

UG Chambers - Invert elev. = 209.00 ft, Rise x Span = 1.00 x 3.00 ft, Barrel Len = 32.00 ft, No. Barrels = 1, Slope = 0.00%, Headers = No Encasement - Invert elev. = 208.50 ft, Width = 5.00 ft, Height = 2.00 ft, Voids = 40.00%

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	208.50	n/a	0	0
0.20	208.70	n/a	13	13
0.40	208.90	n/a	13	26
0.60	209.10	n/a	19	44
0.80	209.30	n/a	24	68
1.00	209.50	n/a	23	92
1.20	209.70	n/a	22	114
1.40	209.90	n/a	20	133
1.60	210.10	n/a	14	148
1.80	210.30	n/a	13	160
2.00	210.50	n/a	13	173

Culvert / Orifice Structures

	[A]	[B]	[C]	[PrfRsr]		[A]	[B]	[C]	[D]
Rise (in)	= 0.00	0.00	0.00	0.00	Crest Len (ft)	= 20.00	0.00	0.00	0.00
Span (in)	= 0.00	0.00	0.00	0.00	Crest El. (ft)	= 210.40	0.00	0.00	0.00
No. Barrels	= 0	0	0	0	Weir Coeff.	= 2.60	3.33	3.33	3.33
Invert El. (ft)	= 0.00	0.00	0.00	0.00	Weir Type	= Broad			
Length (ft)	= 0.00	0.00	0.00	0.00	Multi-Stage	= No	No	No	No
Slope (%)	= 0.00	0.00	0.00	n/a	-				
N-Value	= .013	.013	.013	n/a					
Orifice Coeff.	= 0.60	0.60	0.60	0.60	Exfil.(in/hr)	= 1.020 (by	Wet area)		
Multi-Stage	= n/a	No	No	No	TW Elev. (ft)	= 0.00	,		

Weir Structures

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).

Stage / Discharge Stage (ft) Elev (ft) 3.00 211.50 2.00 210.50 1.00 209.50 0.00 208.50 2.00 0.00 0.50 1.00 1.50 Discharge (cfs)

Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 6

Chamber Sys B

Hydrograph type	= Reservoir	Peak discharge	= 0.000 cfs
Storm frequency	= 2 yrs	Time to peak	= 605 min
Time interval	= 1 min	Hyd. volume	= 0 cuft
Inflow hyd. No.	= 4 - P-3	Max. Elevation	= 229.47 ft
Reservoir name	= RA-B	Max. Storage	= 126 cuft

Storage Indication method used. Exfiltration extracted from Outflow.



Pond Report

Hydraflow Hydrographs by Intelisolve v9.2

Pond No. 2 - RA-B

Pond Data

UG Chambers - Invert elev. = 229.00 ft, Rise x Span = 1.00×3.00 ft, Barrel Len = 23.00 ft, No. Barrels = 2, Slope = 0.00%, Headers = No **Encasement -** Invert elev. = 228.50 ft, Width = 5.00 ft, Height = 2.00 ft, Voids = 40.00%

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	228.50	n/a	0	0
0.20	228.70	n/a	18	18
0.40	228.90	n/a	18	37
0.60	229.10	n/a	27	63
0.80	229.30	n/a	35	98
1.00	229.50	n/a	34	132
1.20	229.70	n/a	32	163
1.40	229.90	n/a	28	191
1.60	230.10	n/a	21	212
1.80	230.30	n/a	18	231
2.00	230.50	n/a	18	249

Culvert / Orifice Structures

	[A]	[B]	[C]	[PrfRsr]		[A]	[B]	[C]	[D]	
Rise (in)	= 0.00	0.00	0.00	0.00	Crest Len (ft)	= 20.00	0.00	0.00	0.00	
Span (in)	= 0.00	0.00	0.00	0.00	Crest El. (ft)	= 230.45	0.00	0.00	0.00	
No. Barrels	= 0	0	0	0	Weir Coeff.	= 2.60	3.33	3.33	3.33	
Invert El. (ft)	= 0.00	0.00	0.00	0.00	Weir Type	= Broad				
Length (ft)	= 0.00	0.00	0.00	0.00	Multi-Stage	= No	No	No	No	
Slope (%)	= 0.00	0.00	0.00	n/a	-					
N-Value	= .013	.013	.013	n/a						
Orifice Coeff.	= 0.60	0.60	0.60	0.60	Exfil.(in/hr)	= 1.020 (by	Wet area)			
Multi-Stage	= n/a	No	No	No	TW Elev. (ft)	= 0.00				

Weir Structures

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).



Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 7

Flow to SP-A

Hydrograph type	= Combine	Peak discharge	= 0.000 cfs
Storm frequency	= 2 yrs	Time to peak	= 1324 min
Time interval	= 1 min	Hyd. volume	= 3 cuft
Inflow hyds.	= 2, 5, 6	Contrib. drain. area	= 0.105 ac



Hydrograph Summary Report

Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph description
1	SCS Runoff	0.010	1	744	127				E-1
2	SCS Runoff	0.005	1	746	76				P-1
3	SCS Runoff	0.068	1	724	239				P-2
4	SCS Runoff	0.136	1	724	479				P-3
5	Reservoir	0.000	1	599	0	3	209.53	94.4	Chamber Sys A
6	Reservoir	0.000	1	424	0	4	230.09	211	Chamber Sys B
7	Combine	0.005	1	746	76	2, 5, 6			Flow to SP-A
					Poturo	eriod: 10 V		Monday Er	
hydro Henry St.gpw Return Period: 10 Year Monday, Feb 1, 2021					eb 1, 2021				

Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 1

E-1

Hydrograph type	= SCS Runoff	Peak discharge	= 0.010 cfs
Storm frequency	= 10 yrs	Time to peak	= 744 min
Time interval	= 1 min	Hyd. volume	= 127 cuft
Drainage area	= 0.150 ac	Curve number	= 43
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= USER	Time of conc. (Tc)	= 6.00 min
Total precip.	= 4.50 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484



Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 2

P-1

Hydrograph type	= SCS Runoff	Peak discharge	= 0.005 cfs
Storm frequency	= 10 yrs	Time to peak	= 746 min
Time interval	= 1 min	Hyd. volume	= 76 cuft
Drainage area	= 0.105 ac	Curve number	= 42
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= USER	Time of conc. (Tc)	= 6.00 min
Total precip.	= 4.50 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484



Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 3

P-2

Hydrograph type	= SCS Runoff	Peak discharge	= 0.068 cfs
Storm frequency	= 10 yrs	Time to peak	= 724 min
Time interval	= 1 min	Hyd. volume	= 239 cuft
Drainage area	= 0.015 ac	Curve number	= 98
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= USER	Time of conc. (Tc)	= 6.00 min
Total precip.	= 4.50 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484



Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 4

P-3

Hydrograph type	= SCS Runoff	Peak discharge	= 0.136 cfs
Storm frequency	= 10 yrs	Time to peak	= 724 min
Time interval	= 1 min	Hyd. volume	= 479 cuft
Drainage area	= 0.030 ac	Curve number	= 98
Basin Šlope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= USER	Time of conc. (Tc)	= 6.00 min
Total precip.	= 4.50 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484



Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 5

Chamber Sys A

Hydrograph type	= Reservoir	Peak discharge	= 0.000 cfs
Storm frequency	= 10 yrs	Time to peak	= 599 min
Time interval	= 1 min	Hyd. volume	= 0 cuft
Inflow hyd. No.	= 3 - P-2	Max. Elevation	= 209.53 ft
Reservoir name	= RA-B	Max. Storage	= 94 cuft

Storage Indication method used. Exfiltration extracted from Outflow.



Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 6

Chamber Sys B

Reservoir	Peak discharge	= 0.000 cfs
⊧ 10 yrs	Time to peak	= 424 min
1 min	Hyd. volume	= 0 cuft
= 4 - P-3	Max. Elevation	= 230.09 ft
RA-B	Max. Storage	= 211 cuft
	Reservoir 10 yrs 1 min 4 - P-3 RA-B	ReservoirPeak discharge10 yrsTime to peak1 minHyd. volume4 - P-3Max. ElevationRA-BMax. Storage

Storage Indication method used. Exfiltration extracted from Outflow.



Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 7

Flow to SP-A

Hydrograph type	= Combine	Peak discharge	= 0.005 cfs
Storm frequency	= 10 yrs	Time to peak	= 746 min
Time interval	= 1 min	Hyd. volume	= 76 cuft
Inflow hyds.	= 2, 5, 6	Contrib. drain. area	= 0.105 ac



Hydrograph Summary Report

Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph description
1	SCS Runoff	0.030	1	738	248				E-1
2	SCS Runoff	0.017	1	740	155				P-1
3	SCS Runoff	0.081	1	724	284				P-2
4	SCS Runoff	0.161	1	724	569				P-3
5	Reservoir	0.000	1	554	0	3	209.75	118	Chamber Sys A
6	Reservoir	0.019	1	751	19	4	230.45	244	Chamber Sys B
7	Combine	0.031	1	751	174	2, 5, 6			Flow to SP-A
hyd	ro Henry St.g	pw			Return P	eriod: 25 Y	ear	Monday, Fe	eb 1, 2021

Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 1

E-1

Hydrograph type	= SCS Runoff	Peak discharge	= 0.030 cfs
Storm frequency	= 25 yrs	Time to peak	= 738 min
Time interval	= 1 min	Hyd. volume	= 248 cuft
Drainage area	= 0.150 ac	Curve number	= 43
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= USER	Time of conc. (Tc)	= 6.00 min
Total precip.	= 5.30 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484



Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 2

P-1

Hydrograph type	= SCS Runoff	Peak discharge	= 0.017 cfs
Storm frequency	= 25 yrs	Time to peak	= 740 min
Time interval	= 1 min	Hyd. volume	= 155 cuft
Drainage area	= 0.105 ac	Curve number	= 42
Basin Šlope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= USER	Time of conc. (Tc)	= 6.00 min
Total precip.	= 5.30 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484



Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 3

P-2

Hydrograph type	= SCS Runoff	Peak discharge	= 0.081 cfs
Storm frequency	= 25 yrs	Time to peak	= 724 min
Time interval	= 1 min	Hyd. volume	= 284 cuft
Drainage area	= 0.015 ac	Curve number	= 98
Basin Šlope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= USER	Time of conc. (Tc)	= 6.00 min
Total precip.	= 5.30 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484



Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 4

P-3

Hydrograph type	= SCS Runoff	Peak discharge	= 0.161 cfs
Storm frequency	= 25 yrs	Time to peak	= 724 min
Time interval	= 1 min	Hyd. volume	= 569 cuft
Drainage area	= 0.030 ac	Curve number	= 98
Basin Šlope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= USER	Time of conc. (Tc)	= 6.00 min
Total precip.	= 5.30 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484



Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 5

Chamber Sys A

= Reservoir	Peak discharge	= 0.000 cfs
= 25 yrs	Time to peak	= 554 min
= 1 min	Hyd. volume	= 0 cuft
= 3 - P-2	Max. Elevation	= 209.75 ft
= RA-B	Max. Storage	= 118 cuft
	= Reservoir = 25 yrs = 1 min = 3 - P-2 = RA-B	= ReservoirPeak discharge= 25 yrsTime to peak= 1 minHyd. volume= 3 - P-2Max. Elevation= RA-BMax. Storage

Storage Indication method used. Exfiltration extracted from Outflow.



Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 6

Chamber Sys B

Hydrograph type	= Reservoir	Peak discharge	= 0.019 cfs
Storm frequency	= 25 yrs	Time to peak	= 751 min
Time interval	= 1 min	Hyd. volume	= 19 cuft
Inflow hyd. No.	= 4 - P-3	Max. Elevation	= 230.45 ft
Reservoir name	= RA-B	Max. Storage	= 244 cuft

Storage Indication method used. Exfiltration extracted from Outflow.



Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 7

Flow to SP-A

Hydrograph type	= Combine	Peak discharge	= 0.031 cfs
Storm frequency	= 25 yrs	Time to peak	= 751 min
Time interval	= 1 min	Hyd. volume	= 174 cuft
Inflow hyds.	= 2, 5, 6	Contrib. drain. area	= 0.105 ac



Hydrograph Summary Report

Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph description
1	SCS Runoff	0.135	1	726	603				E-1
2	SCS Runoff	0.082	1	727	391				P-1
3	SCS Runoff	0.107	1	724	380				P-2
4	SCS Runoff	0.213	1	724	759				P-3
5	Reservoir	0.005	1	767	5	3	210.40	167	Chamber Sys A
6	Reservoir	0.221	1	728	146	4	230.50	247	Chamber Sys B
7	Combine	0.301	1	728	541	2, 5, 6			Flow to SP-A
hyd	ro Henry St.g	pw			Return P	eriod: 100	Year	Monday, Fe	eb 1, 2021

Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 1

E-1

Hydrograph type	= SCS Runoff	Peak discharge	= 0.135 cfs
Storm frequency	= 100 yrs	Time to peak	= 726 min
Time interval	= 1 min	Hyd. volume	= 603 cuft
Drainage area	= 0.150 ac	Curve number	= 43
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= USER	Time of conc. (Tc)	= 6.00 min
Total precip.	= 7.00 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484



Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 2

P-1

Hydrograph type	= SCS Runoff	Peak discharge	= 0.082 cfs
Storm frequency	= 100 yrs	Time to peak	= 727 min
Time interval	= 1 min	Hyd. volume	= 391 cuft
Drainage area	= 0.105 ac	Curve number	= 42
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= USER	Time of conc. (Tc)	= 6.00 min
Total precip.	= 7.00 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484



Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 3

P-2

Hydrograph type	= SCS Runoff	Peak discharge	= 0.107 cfs
Storm frequency	= 100 yrs	Time to peak	= 724 min
Time interval	= 1 min	Hyd. volume	= 380 cuft
Drainage area	= 0.015 ac	Curve number	= 98
Basin Šlope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= USER	Time of conc. (Tc)	= 6.00 min
Total precip.	= 7.00 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484



Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 4

P-3

Hydrograph type	= SCS Runoff	Peak discharge	= 0.213 cfs
Storm frequency	= 100 yrs	Time to peak	= 724 min
Time interval	= 1 min	Hyd. volume	= 759 cuft
Drainage area	= 0.030 ac	Curve number	= 98
Basin Šlope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= USER	Time of conc. (Tc)	= 6.00 min
Total precip.	= 7.00 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484



Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 5

Chamber Sys A

Hydrograph type	= Reservoir	Peak discharge	= 0.005 cfs
Storm frequency	= 100 yrs	Time to peak	= 767 min
Time interval	= 1 min	Hyd. volume	= 5 cuft
Inflow hyd. No.	= 3 - P-2	Max. Elevation	= 210.40 ft
Reservoir name	= RA-B	Max. Storage	= 167 cuft
Time interval Inflow hyd. No. Reservoir name	= 1 min = 3 - P-2 = RA-B	Hyd. volume Max. Elevation Max. Storage	= 5 cuft = 210.40 ft = 167 cuft

Storage Indication method used. Exfiltration extracted from Outflow.



Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 6

Chamber Sys B

Hydrograph type	= Reservoir	Peak discharge	= 0.221 cfs
Storm frequency	= 100 yrs	Time to peak	= 728 min
Time interval	= 1 min	Hyd. volume	= 146 cuft
Inflow hyd. No.	= 4 - P-3	Max. Elevation	= 230.50 ft
Reservoir name	= RA-B	Max. Storage	= 247 cuft

Storage Indication method used. Exfiltration extracted from Outflow.



Hydraflow Hydrographs by Intelisolve v9.2

Hyd. No. 7

Flow to SP-A

Hydrograph type	= Combine	Peak discharge	= 0.301 cfs
Storm frequency	= 100 yrs	Time to peak	= 728 min
Time interval	= 1 min	Hyd. volume	= 541 cuft
Inflow hyds.	= 2, 5, 6	Contrib. drain. area	= 0.105 ac



RECHARGE AREA CHARACTERISTICS

Pond Report

Hydraflow Hydrographs by Intelisolve v9.2

Pond No. 1 - RA-B

Pond Data

UG Chambers - Invert elev. = 209.00 ft, Rise x Span = 1.00 x 3.00 ft, Barrel Len = 32.00 ft, No. Barrels = 1, Slope = 0.00%, Headers = No Encasement - Invert elev. = 208.50 ft, Width = 5.00 ft, Height = 2.00 ft, Voids = 40.00%

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	208.50	n/a	0	0
0.20	208.70	n/a	13	13
0.40	208.90	n/a	13	26
0.60	209.10	n/a	19	44
0.80	209.30	n/a	24	68
1.00	209.50	n/a	23	92
1.20	209.70	n/a	22	114
1.40	209.90	n/a	20	133
1.60	210.10	n/a	14	148
1.80	210.30	n/a	13	160
2.00	210.50	n/a	13	173

Culvert / Orifice Structures

	[A]	[B]	[C]	[PrfRsr]		[A]	[B]	[C]	[D]
Rise (in)	= 0.00	0.00	0.00	0.00	Crest Len (ft)	= 20.00	0.00	0.00	0.00
Span (in)	= 0.00	0.00	0.00	0.00	Crest El. (ft)	= 210.40	0.00	0.00	0.00
No. Barrels	= 0	0	0	0	Weir Coeff.	= 2.60	3.33	3.33	3.33
Invert El. (ft)	= 0.00	0.00	0.00	0.00	Weir Type	= Broad			
Length (ft)	= 0.00	0.00	0.00	0.00	Multi-Stage	= No	No	No	No
Slope (%)	= 0.00	0.00	0.00	n/a	-				
N-Value	= .013	.013	.013	n/a					
Orifice Coeff.	= 0.60	0.60	0.60	0.60	Exfil.(in/hr)	= 1.020 (by	Wet area)		
Multi-Stage	= n/a	No	No	No	TW Elev. (ft)	= 0.00	,		

Weir Structures

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).

Stage / Discharge Stage (ft) Elev (ft) 3.00 211.50 2.00 210.50 1.00 209.50 0.00 208.50 2.00 0.00 0.50 1.00 1.50 Discharge (cfs)

Pond Report

Hydraflow Hydrographs by Intelisolve v9.2

Pond No. 2 - RA-B

Pond Data

UG Chambers - Invert elev. = 229.00 ft, Rise x Span = 1.00×3.00 ft, Barrel Len = 23.00 ft, No. Barrels = 2, Slope = 0.00%, Headers = No **Encasement -** Invert elev. = 228.50 ft, Width = 5.00 ft, Height = 2.00 ft, Voids = 40.00%

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	228.50	n/a	0	0
0.20	228.70	n/a	18	18
0.40	228.90	n/a	18	37
0.60	229.10	n/a	27	63
0.80	229.30	n/a	35	98
1.00	229.50	n/a	34	132
1.20	229.70	n/a	32	163
1.40	229.90	n/a	28	191
1.60	230.10	n/a	21	212
1.80	230.30	n/a	18	231
2.00	230.50	n/a	18	249

Culvert / Orifice Structures

	[A]	[B]	[C]	[PrfRsr]		[A]	[B]	[C]	[D]
Rise (in)	= 0.00	0.00	0.00	0.00	Crest Len (ft)	= 20.00	0.00	0.00	0.00
Span (in)	= 0.00	0.00	0.00	0.00	Crest El. (ft)	= 230.45	0.00	0.00	0.00
No. Barrels	= 0	0	0	0	Weir Coeff.	= 2.60	3.33	3.33	3.33
Invert El. (ft)	= 0.00	0.00	0.00	0.00	Weir Type	= Broad			
Length (ft)	= 0.00	0.00	0.00	0.00	Multi-Stage	= No	No	No	No
Slope (%)	= 0.00	0.00	0.00	n/a	-				
N-Value	= .013	.013	.013	n/a					
Orifice Coeff.	= 0.60	0.60	0.60	0.60	Exfil.(in/hr)	= 1.020 (by Wet area)			
Multi-Stage	= n/a	No	No	No	TW Elev. (ft)	= 0.00	,		

Weir Structures

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).



Operation and Maintenance Plan

This Stormwater Operation and Maintenance Plan covers the post-construction operation and maintenance of the stormwater management system for 25 Henry St. in Arlington, Massachusetts.

The procedures, practices, and schedule outlined in this plan are intended to be ongoing requirements and are an important factor in ensuring the continued proper functioning of the stormwater management system and integrity of the discharged stormwater.

The following maintenance requirements are the sole responsibility of the property owner(s).

Trench Grate

- Remove leaf litter and other debris from grate as needed to ensure adequate capacity for collection of runoff.
- Inspect system at least four times per year and clean sump when debris is accumulated to a depth of six inches.

Recharge Systems

- Inspect systems after every major storm in the first three months of construction to ensure proper stabilization and function. Thereafter, inspect quarterly.
- Clean systems at least once per year, or more frequently, as needed to prevent accumulation of sediment and other debris in the system.
- Clean structures when debris is accumulated to a depth of 6 inches.