

ALAN ENGINEERING, L.L.C.

288 Littleton Road, Suite 31
Westford, MA 01886
(978) 577-6444
alan.eng@verizon.net

June 28, 2016

Scott Seaver
Seaver Construction, Inc.
215 Lexington Street
Woburn, MA 01801

Ref: Drainage Analysis
47 Spy Pond Lane – Lot 1
Arlington, MA

Dear Mr. Seaver:

Alan Engineering has prepared the following drainage analysis of the proposed house on Lot 1 at 47 Spy Pond Lane in Arlington, MA.

This analysis compares runoff generated from the existing site to the runoff that will be generated from the site after the construction of the new house. In accordance with the requirements of the Arlington Conservation Commission the 10-year, 25-year, and 100-year storm events were analyzed. The storm events were 24-hour rainfalls with a Type III rainfall distribution. The rainfall amounts were based on the "Cornell Study".

The proposed lot will contain 8,456 square feet of land. Under the existing conditions the lot contains 1,775 square feet of impervious area. The proposed site will contain a total of 2,659 square feet of impervious area.

The increase in impervious area will result in an increase in the rate and volume of runoff. In order to mitigate the increase a subsurface roof drain infiltration system is proposed. A roof gutter and downspout system will collect all roof runoff and discharge it into a subsurface system located at the rear of the proposed house. The system will collect and recharge a portion of the roof runoff that is slightly greater than the increase in runoff volume generated by the proposed site development. The result is a decrease in both the peak rate and total volume of runoff from the site. The results of the analysis are summarized in the table below.

Test pits were excavated on the lot on June 28, 2016 to determine the permeability of the soil and the depth to groundwater. All test pits had approximately 5 feet of fill above the original ground. The underlying native soil is fine sand. A percolation test yielded a rate of 1 minute per inch. This is indicative of hydrologic soil group (HSG) A. The estimated seasonal high groundwater ranged from 54 inches to 66 inches below the ground surface in 3 of the 4 test holes, and 90 inches below the ground surface in the higher of the 4 test holes.

Comparative Hydrologic Summary
47 Spy Pond Lane - Lot 1
 Arlington, MA
 June 28, 2016

10 Year Storm - 4.80 inches

Point of Analysis	Pre-Development		Post Development	
	Peak Rate (c.f.s.)	Volume (ac-ft)	Peak Rate (c.f.s.)	Volume (ac-ft)
Total Runoff	0.03	0.006	0.01	0.003

50 Year Storm - 7.06 inches

Point of Analysis	Pre-Development		Post Development	
	Peak Rate (c.f.s.)	Volume (ac-ft)	Peak Rate (c.f.s.)	Volume (ac-ft)
Total Runoff	0.23	0.020	0.12	0.013

100 Year Storm - 8.48 inches

Point of Analysis	Pre-Development		Post Development	
	Peak Rate (c.f.s.)	Volume (ac-ft)	Peak Rate (c.f.s.)	Volume (ac-ft)
Total Runoff	0.41	0.032	0.24	0.022

Please feel free to contact me with any questions or comments.

Very truly yours,

ALAN ENGINEERING, L.L.C.



Mark A. Sleger, P.E.
 Manager

ALAN ENGINEERING, L.L.C.

SOIL EVALUATION REPORT

Job Number 1140 Client SEAUER CONSTRUCTION
 Site Address 47 SPY POND LANE Town ARLINGTON

Current Use RESIDENTIAL
 Site Description SINGLE FAMILY RESIDENCE
 Land Form GROUND MORRAINE
 Vegetation LAWN
 Water Supply TOWN

Deep Hole No AE-1 Date 6/28/2016
 Soil Evaluator M. SLEGER Temperature 65°
 Local Official N/A Weather CLOUDY - LIGHT RAIN

Horizon	Depth	Classification	Color	Comments		
FILL	0-54"	SANDY LOAM	—	SOME GRAVEL		
C	54-126	FINE SAND	10YR 5/4			
Seepage	Standing	Mottling	Color	ESHW	Roots	Refusal
108"	—	60"	2.5Y 6/3	60"	72"	—

Deep Hole No AE-2

Horizon	Depth	Classification	Color	Comments		
FILL	0-60"	SANDY FILL	—	MOTTLING IN SAND FILL		
A	60-69"	SANDY LOAM	10YR 2/2			
B	69-78"	FINE SAND	10YR 4/6			
C	78-120	FINE SAND	10YR 5/4			
Seepage	Standing	Mottling	Color	ESHW	Roots	Refusal
108"	108"	54"		54"	78"	—

Deep Hole No AE-3

Horizon	Depth	Classification	Color	Comments		
FILL	0-60"	SANDY FILL	—			
C ₁	60-138"	FINE SAND	10YR 5/4			
Seepage	Standing	Mottling	Color	ESHW	Roots	Refusal
—	—	90"	2.5Y 6/3	90"	96"	—

Deep Hole No AE-4

Horizon	Depth	Classification	Color	Comments		
FILL	0-66"	SANDY FILL				
C ₁	66-114"	FINE SAND	10YR 5/4			
Seepage	Standing	Mottling	Color	ESHW	Roots	Refusal
—	—	66"	2.5Y 6/3	66"		

ALAN ENGINEERING, L.L.C.

SOIL EVALUATION REPORT

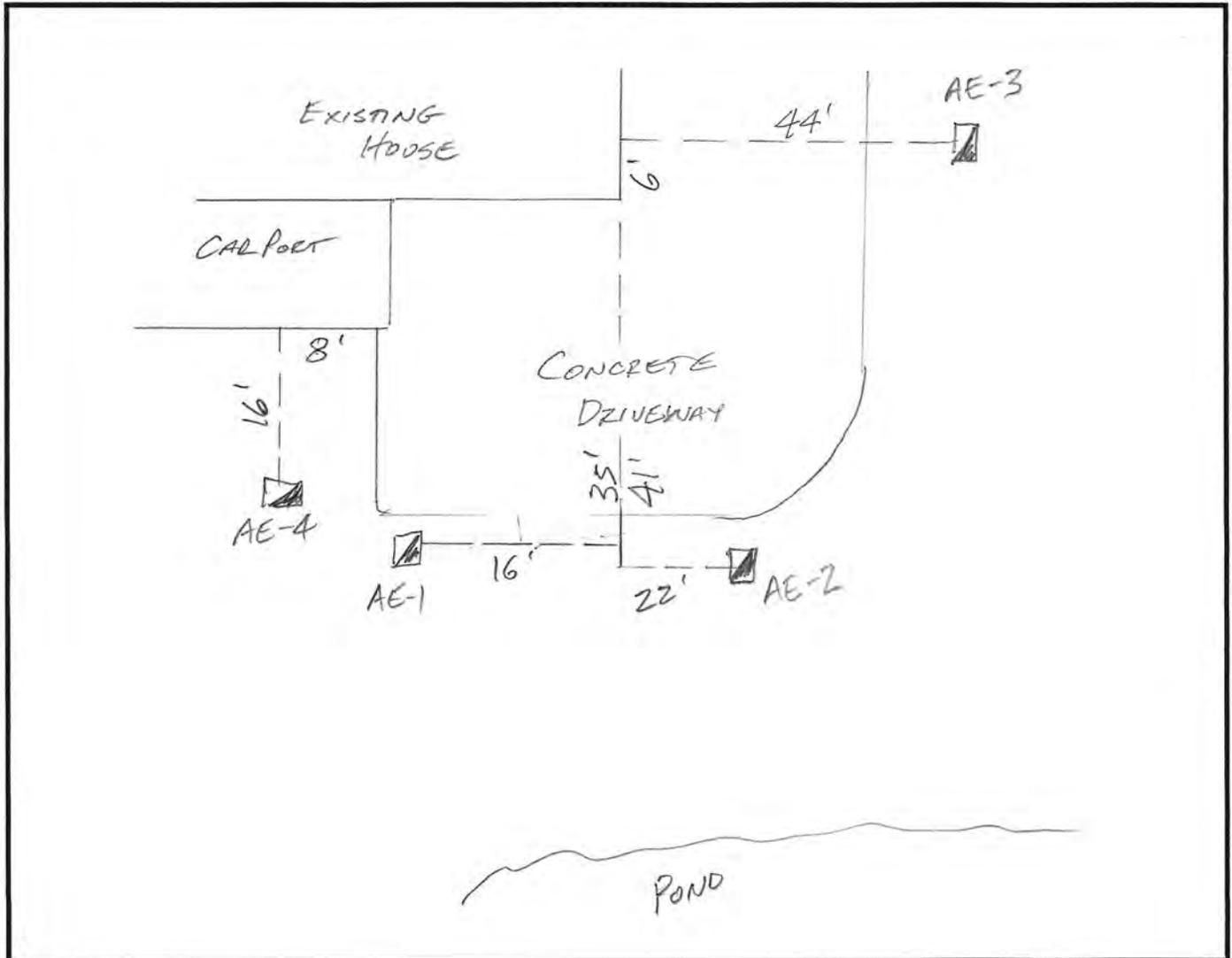
Job Number 1140 Client SCAVER CONST.
 Site Address 47 SPY POND LANE Town ARLINGTON

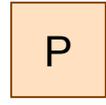
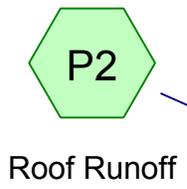
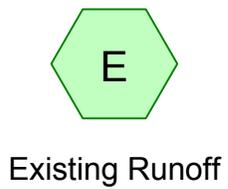
PERCOLATION TESTS

Soil Evaluator M. SLEGER Date 6/28/2016 Temperature 65°
 Local Official N/A Weather LIGHT RAIN

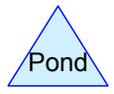
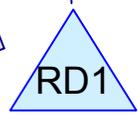
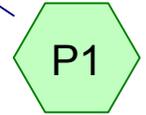
Deep Hole No	AE-1			
Depth to Bottom	84"			
Soil Classification	FINE SAND			
Start Pre Soak	9:02			
Start of Test - 12"	9:17			
Time at 9"	9:21			
Time at 6"	9:24			
Time from 9" to 6"	3 MIN			
Percolation Rate	1 MIN/INCH			

SITE SKETCH





Total Proposed Runoff



Lot 1 Drainage Analysis

Prepared by ALAN Engineering, L.L.C.

HydroCAD® 10.00-16 s/n 04219 © 2015 HydroCAD Software Solutions LLC

Runoff Comparison - Lot 1

Type III 24-hr 10-Year Storm Rainfall=4.80"

Page 2

Summary for Subcatchment E: Existing Runoff

Runoff = 0.03 cfs @ 12.31 hrs, Volume= 0.006 af, Depth> 0.38"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-Year Storm Rainfall=4.80"

Area (sf)	CN	Adj	Description
1,775	98		Unconnected pavement, HSG A
6,681	39		>75% Grass cover, Good, HSG A
8,456	51	45	Weighted Average, UI Adjusted
6,681			79.01% Pervious Area
1,775			20.99% Impervious Area
1,775			100.00% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

Summary for Subcatchment P1: Proposed Yard Runoff

Runoff = 0.01 cfs @ 12.39 hrs, Volume= 0.003 af, Depth> 0.26"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-Year Storm Rainfall=4.80"

Area (sf)	CN	Adj	Description
692	98		Unconnected pavement, HSG A
5,797	39		>75% Grass cover, Good, HSG A
6,489	45	42	Weighted Average, UI Adjusted
5,797			89.34% Pervious Area
692			10.66% Impervious Area
692			100.00% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

Summary for Subcatchment P2: Roof Runoff

Runoff = 0.22 cfs @ 12.07 hrs, Volume= 0.017 af, Depth> 4.56"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-Year Storm Rainfall=4.80"

Lot 1 Drainage Analysis

Runoff Comparison - Lot 1
 Type III 24-hr 10-Year Storm Rainfall=4.80"

Prepared by ALAN Engineering, L.L.C.

HydroCAD® 10.00-16 s/n 04219 © 2015 HydroCAD Software Solutions LLC

Page 3

Area (sf)	CN	Description
1,967	98	Roofs, HSG A
1,967		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

Summary for Reach P: Total Proposed Runoff

Inflow Area = 0.194 ac, 31.45% Impervious, Inflow Depth > 0.20" for 10-Year Storm event
 Inflow = 0.01 cfs @ 12.39 hrs, Volume= 0.003 af
 Outflow = 0.01 cfs @ 12.39 hrs, Volume= 0.003 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Summary for Pond RD1: Roof Drain System

Inflow Area = 0.045 ac, 100.00% Impervious, Inflow Depth > 4.56" for 10-Year Storm event
 Inflow = 0.22 cfs @ 12.07 hrs, Volume= 0.017 af
 Outflow = 0.05 cfs @ 11.73 hrs, Volume= 0.017 af, Atten= 77%, Lag= 0.0 min
 Discarded = 0.05 cfs @ 11.73 hrs, Volume= 0.017 af
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
 Peak Elev= 7.49' @ 12.45 hrs Surf.Area= 262 sf Storage= 148 cf

Plug-Flow detention time= 13.8 min calculated for 0.017 af (100% of inflow)
 Center-of-Mass det. time= 13.7 min (761.0 - 747.3)

Volume	Invert	Avail.Storage	Storage Description
#1A	6.50'	198 cf	11.25'W x 23.25'L x 2.54'H Field A 665 cf Overall - 169 cf Embedded = 496 cf x 40.0% Voids
#2A	7.00'	169 cf	Cultec R-150XLHD x 6 Inside #1 Effective Size= 29.8"W x 18.0"H => 2.65 sf x 10.25'L = 27.2 cf Overall Size= 33.0"W x 18.5"H x 11.00'L with 0.75' Overlap Row Length Adjustment= +0.75' x 2.65 sf x 3 rows
		367 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	6.50'	8.270 in/hr Exfiltration over Horizontal area
#2	Primary	8.50'	4.0" Round Culvert X 2.00 L= 5.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 8.50' / 8.40' S= 0.0200 1' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.09 sf

Lot 1 Drainage Analysis

Prepared by ALAN Engineering, L.L.C.

HydroCAD® 10.00-16 s/n 04219 © 2015 HydroCAD Software Solutions LLC

Runoff Comparison - Lot 1

Type III 24-hr 10-Year Storm Rainfall=4.80"

Page 4

Discarded OutFlow Max=0.05 cfs @ 11.73 hrs HW=6.53' (Free Discharge)

↑**1=Exfiltration** (Exfiltration Controls 0.05 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=6.50' (Free Discharge)

↑**2=Culvert** (Controls 0.00 cfs)

Lot 1 Drainage Analysis

Prepared by ALAN Engineering, L.L.C.

HydroCAD® 10.00-16 s/n 04219 © 2015 HydroCAD Software Solutions LLC

Runoff Comparison - Lot 1

Type III 24-hr 50-Year Storm Rainfall=7.06"

Page 5

Summary for Subcatchment E: Existing Runoff

Runoff = 0.23 cfs @ 12.10 hrs, Volume= 0.020 af, Depth> 1.26"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 50-Year Storm Rainfall=7.06"

Area (sf)	CN	Adj	Description
1,775	98		Unconnected pavement, HSG A
6,681	39		>75% Grass cover, Good, HSG A
8,456	51	45	Weighted Average, UI Adjusted
6,681			79.01% Pervious Area
1,775			20.99% Impervious Area
1,775			100.00% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

Summary for Subcatchment P1: Proposed Yard Runoff

Runoff = 0.12 cfs @ 12.11 hrs, Volume= 0.013 af, Depth> 1.02"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 50-Year Storm Rainfall=7.06"

Area (sf)	CN	Adj	Description
692	98		Unconnected pavement, HSG A
5,797	39		>75% Grass cover, Good, HSG A
6,489	45	42	Weighted Average, UI Adjusted
5,797			89.34% Pervious Area
692			10.66% Impervious Area
692			100.00% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

Summary for Subcatchment P2: Roof Runoff

Runoff = 0.32 cfs @ 12.07 hrs, Volume= 0.026 af, Depth> 6.82"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 50-Year Storm Rainfall=7.06"

Lot 1 Drainage Analysis

Prepared by ALAN Engineering, L.L.C.

HydroCAD® 10.00-16 s/n 04219 © 2015 HydroCAD Software Solutions LLC

Runoff Comparison - Lot 1

Type III 24-hr 50-Year Storm Rainfall=7.06"

Page 6

Area (sf)	CN	Description
1,967	98	Roofs, HSG A
1,967		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

Summary for Reach P: Total Proposed Runoff

Inflow Area = 0.194 ac, 31.45% Impervious, Inflow Depth > 0.78" for 50-Year Storm event
 Inflow = 0.12 cfs @ 12.11 hrs, Volume= 0.013 af
 Outflow = 0.12 cfs @ 12.11 hrs, Volume= 0.013 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Summary for Pond RD1: Roof Drain System

Inflow Area = 0.045 ac, 100.00% Impervious, Inflow Depth > 6.82" for 50-Year Storm event
 Inflow = 0.32 cfs @ 12.07 hrs, Volume= 0.026 af
 Outflow = 0.05 cfs @ 11.64 hrs, Volume= 0.026 af, Atten= 85%, Lag= 0.0 min
 Discarded = 0.05 cfs @ 11.64 hrs, Volume= 0.026 af
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
 Peak Elev= 8.32' @ 12.54 hrs Surf.Area= 262 sf Storage= 289 cf

Plug-Flow detention time= 30.7 min calculated for 0.026 af (100% of inflow)
 Center-of-Mass det. time= 30.6 min (772.0 - 741.5)

Volume	Invert	Avail.Storage	Storage Description
#1A	6.50'	198 cf	11.25'W x 23.25'L x 2.54'H Field A 665 cf Overall - 169 cf Embedded = 496 cf x 40.0% Voids
#2A	7.00'	169 cf	Cultec R-150XLHD x 6 Inside #1 Effective Size= 29.8"W x 18.0"H => 2.65 sf x 10.25'L = 27.2 cf Overall Size= 33.0"W x 18.5"H x 11.00'L with 0.75' Overlap Row Length Adjustment= +0.75' x 2.65 sf x 3 rows
		367 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	6.50'	8.270 in/hr Exfiltration over Horizontal area
#2	Primary	8.50'	4.0" Round Culvert X 2.00 L= 5.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 8.50' / 8.40' S= 0.0200 '/ Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.09 sf

Lot 1 Drainage Analysis

Prepared by ALAN Engineering, L.L.C.

HydroCAD® 10.00-16 s/n 04219 © 2015 HydroCAD Software Solutions LLC

Runoff Comparison - Lot 1

Type III 24-hr 50-Year Storm Rainfall=7.06"

Page 7

Discarded OutFlow Max=0.05 cfs @ 11.64 hrs HW=6.53' (Free Discharge)

↑**1=Exfiltration** (Exfiltration Controls 0.05 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=6.50' (Free Discharge)

↑**2=Culvert** (Controls 0.00 cfs)

Lot 1 Drainage Analysis

Prepared by ALAN Engineering, L.L.C.

HydroCAD® 10.00-16 s/n 04219 © 2015 HydroCAD Software Solutions LLC

Runoff Comparison - Lot 1

Type III 24-hr 100-Year Storm Rainfall=8.48"

Page 8

Summary for Subcatchment E: Existing Runoff

Runoff = 0.41 cfs @ 12.09 hrs, Volume= 0.032 af, Depth> 1.99"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-Year Storm Rainfall=8.48"

Area (sf)	CN	Adj	Description
1,775	98		Unconnected pavement, HSG A
6,681	39		>75% Grass cover, Good, HSG A
8,456	51	45	Weighted Average, UI Adjusted
6,681			79.01% Pervious Area
1,775			20.99% Impervious Area
1,775			100.00% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

Summary for Subcatchment P1: Proposed Yard Runoff

Runoff = 0.24 cfs @ 12.09 hrs, Volume= 0.021 af, Depth> 1.67"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-Year Storm Rainfall=8.48"

Area (sf)	CN	Adj	Description
692	98		Unconnected pavement, HSG A
5,797	39		>75% Grass cover, Good, HSG A
6,489	45	42	Weighted Average, UI Adjusted
5,797			89.34% Pervious Area
692			10.66% Impervious Area
692			100.00% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

Summary for Subcatchment P2: Roof Runoff

Runoff = 0.39 cfs @ 12.07 hrs, Volume= 0.031 af, Depth> 8.23"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-Year Storm Rainfall=8.48"

Lot 1 Drainage Analysis

Runoff Comparison - Lot 1
 Type III 24-hr 100-Year Storm Rainfall=8.48"

Prepared by ALAN Engineering, L.L.C.

HydroCAD® 10.00-16 s/n 04219 © 2015 HydroCAD Software Solutions LLC

Page 9

Area (sf)	CN	Description
1,967	98	Roofs, HSG A
1,967		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

Summary for Reach P: Total Proposed Runoff

Inflow Area = 0.194 ac, 31.45% Impervious, Inflow Depth > 1.38" for 100-Year Storm event
 Inflow = 0.24 cfs @ 12.09 hrs, Volume= 0.022 af
 Outflow = 0.24 cfs @ 12.09 hrs, Volume= 0.022 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Summary for Pond RD1: Roof Drain System

Inflow Area = 0.045 ac, 100.00% Impervious, Inflow Depth > 8.23" for 100-Year Storm event
 Inflow = 0.39 cfs @ 12.07 hrs, Volume= 0.031 af
 Outflow = 0.13 cfs @ 12.32 hrs, Volume= 0.031 af, Atten= 66%, Lag= 15.0 min
 Discarded = 0.05 cfs @ 11.60 hrs, Volume= 0.029 af
 Primary = 0.08 cfs @ 12.32 hrs, Volume= 0.002 af

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
 Peak Elev= 8.65' @ 12.32 hrs Surf.Area= 262 sf Storage= 327 cf

Plug-Flow detention time= 33.1 min calculated for 0.031 af (100% of inflow)
 Center-of-Mass det. time= 33.0 min (772.2 - 739.1)

Volume	Invert	Avail.Storage	Storage Description
#1A	6.50'	198 cf	11.25'W x 23.25'L x 2.54'H Field A 665 cf Overall - 169 cf Embedded = 496 cf x 40.0% Voids
#2A	7.00'	169 cf	Cultec R-150XLHD x 6 Inside #1 Effective Size= 29.8"W x 18.0"H => 2.65 sf x 10.25'L = 27.2 cf Overall Size= 33.0"W x 18.5"H x 11.00'L with 0.75' Overlap Row Length Adjustment= +0.75' x 2.65 sf x 3 rows
		367 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	6.50'	8.270 in/hr Exfiltration over Horizontal area
#2	Primary	8.50'	4.0" Round Culvert X 2.00 L= 5.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 8.50' / 8.40' S= 0.0200 1' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.09 sf

Lot 1 Drainage Analysis

Prepared by ALAN Engineering, L.L.C.

HydroCAD® 10.00-16 s/n 04219 © 2015 HydroCAD Software Solutions LLC

Runoff Comparison - Lot 1

Type III 24-hr 100-Year Storm Rainfall=8.48"

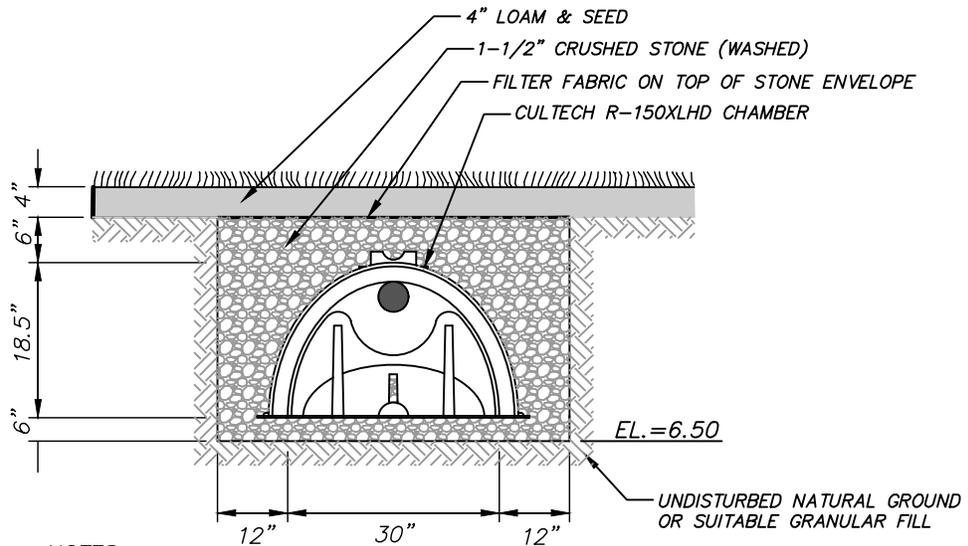
Page 10

Discarded OutFlow Max=0.05 cfs @ 11.60 hrs HW=6.53' (Free Discharge)

↳ **1=Exfiltration** (Exfiltration Controls 0.05 cfs)

Primary OutFlow Max=0.08 cfs @ 12.32 hrs HW=8.65' (Free Discharge)

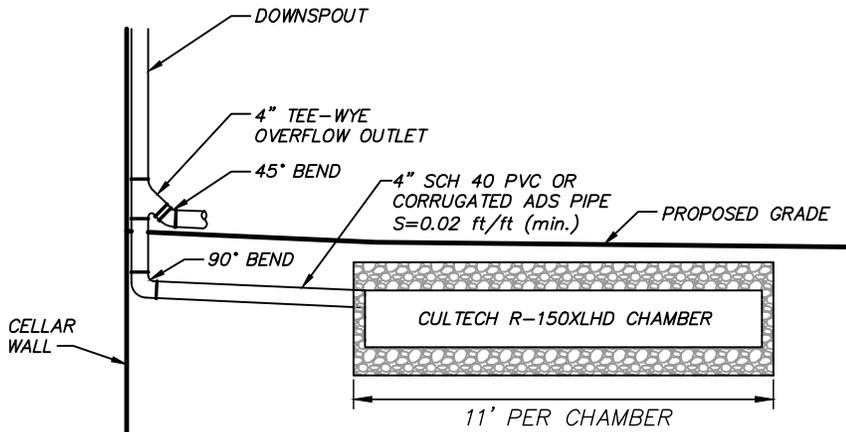
↳ **2=Culvert** (Inlet Controls 0.08 cfs @ 1.06 fps)



NOTES:
 REMOVE ALL TOP AND SUBSOIL AND ANY ORGANIC OR OTHERWISE UNSUITABLE MATERIAL TO A DEPTH OF 2 FEET BENEATH STONE.

ROOF DRAIN LEACHING CHAMBER

NOT TO SCALE

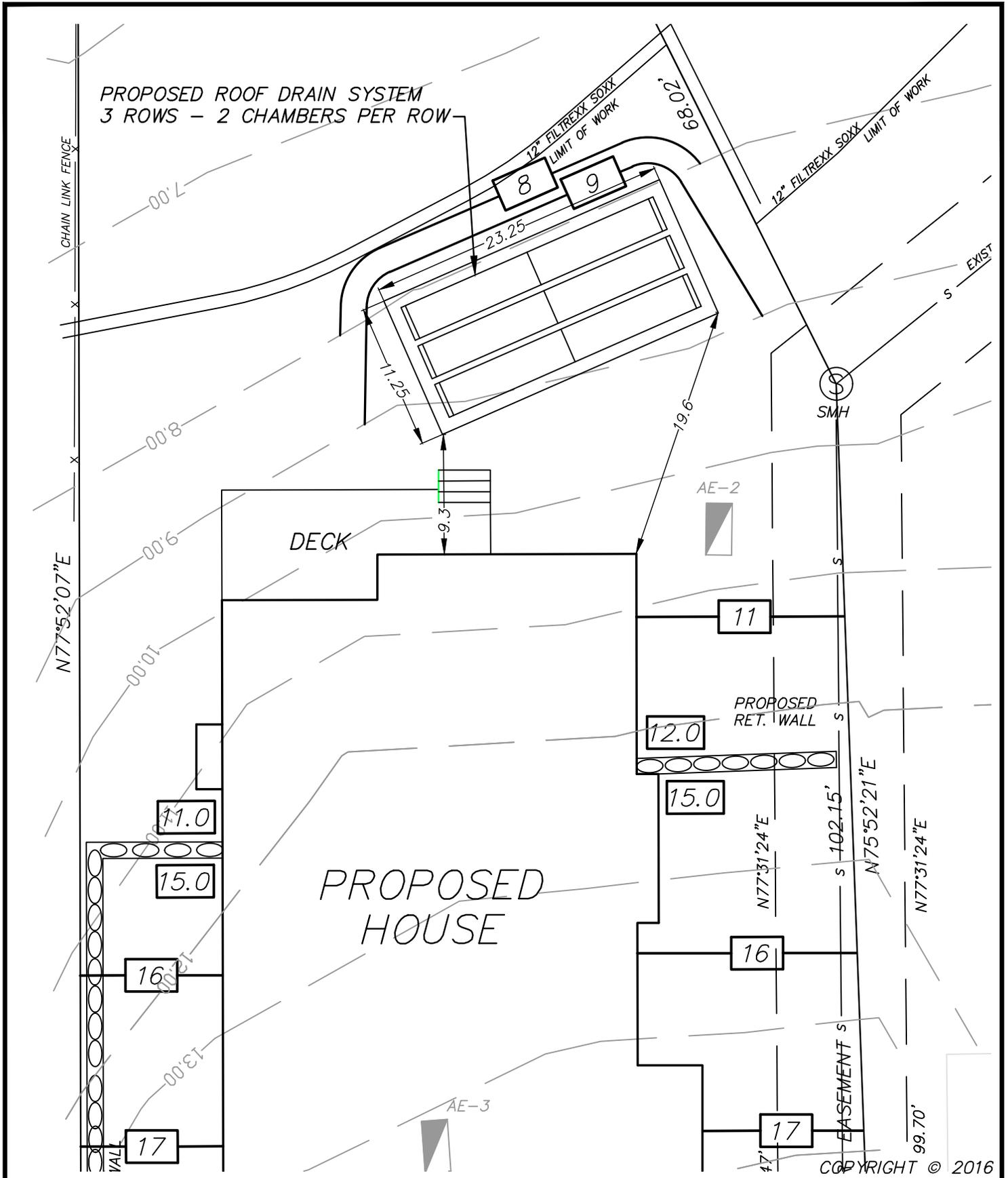


ROOF DRAIN DETAIL

NOT TO SCALE

COPYRIGHT © 2016

ROOF DRAIN DETAIL 47 SPY POND LANE LOT 1 ARLINGTON, MA	ALAN ENGINEERING, L.L.C. 288 LITTLETON ROAD, SUITE 31 WESTFORD, MA 01886	JOB NO. 1140	DWG NO
		JUNE 28, 2016	SHEET 1 of 2
		SCALE: AS SHOWN	



ROOF DRAIN DETAIL
 47 SPY POND LANE
 LOT 1
 ARLINGTON, MA

**ALAN
 ENGINEERING, L.L.C.**
 288 LITTLETON ROAD, SUITE 31
 WESTFORD, MA 01886

JOB NO. 1140
 JUNE 28, 2016
 SCALE: 1" = 10'

DWG NO
 SHEET
 2 of 2

COPYRIGHT © 2016